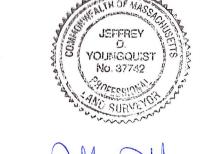


FOR REGISTRY USE ONLY

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DATE PROFESSIONAL LAND SURVEYOR

APPROVED SUBJECT TO TERMS AND CONDITIONS OF A COMPREHENSIVE PERMIT PER ZONING BOARD OF APPEALS DECISION, AS AUTHORIZED BY CHAPTER 40B OF THE MASSACHUSETTS GENERAL LAWS, SECTIONS 20-23.

AC	T	E:		

MEDWAY ZONING BOARD OF APPEALS

THIS PLAN IS SUBJECT TO A COVENANT TO BE RECORDED HEREWITH

I CERTIFY THAT 20 DAYS HAVE ELAPSED SINCE ZONING BOARD APPROVAL, AND THAT NO APPEAL HAS BEEN FILED AT THIS OFFICE.

TOWN CLERK-TOWN OF MEDWAY

PROFILE
KINGSBURY DRIVE
STA 21+50 TO 29+84
HOLLY COURT
& LINDEN PATH

"KINGSBURY VILLAGE' IN MEDWAY MASSACHUSETTS



165 EAST GROVE STREET MIDDLEBOROUGH, MA 02346 TEL: (508)-946-9231 FAX: (508)-947-8873

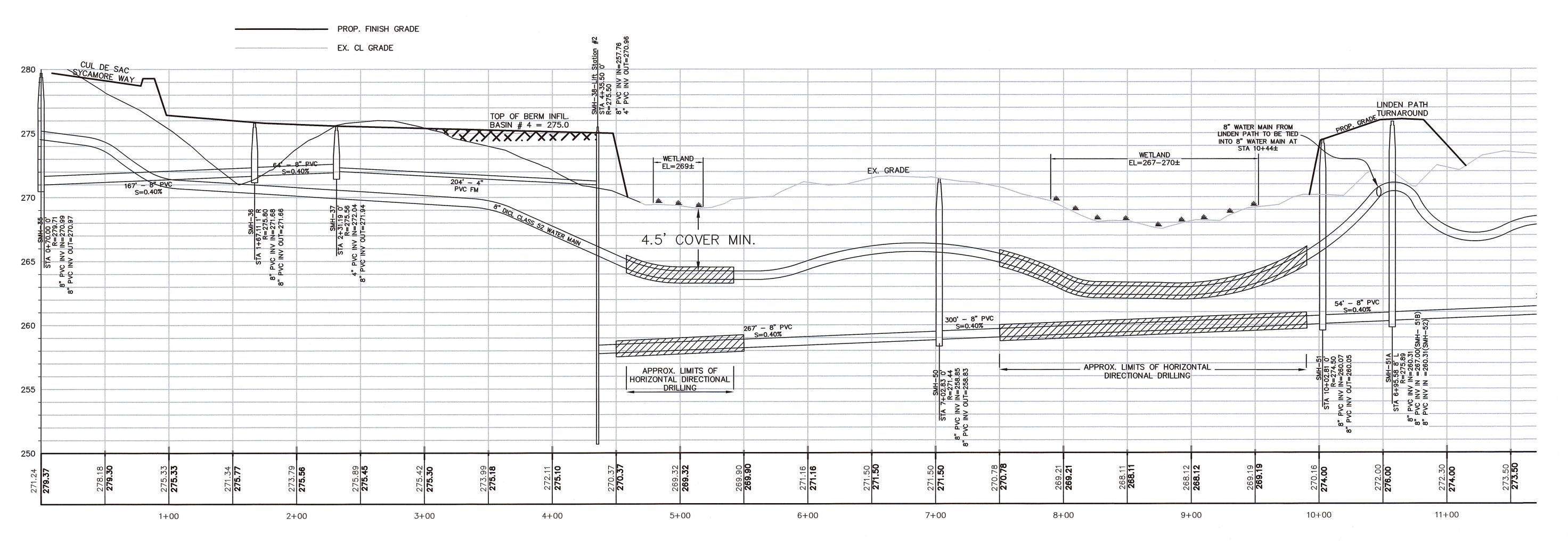
www.outback-eng.com

DATE: NOVEMBER 29, 2019

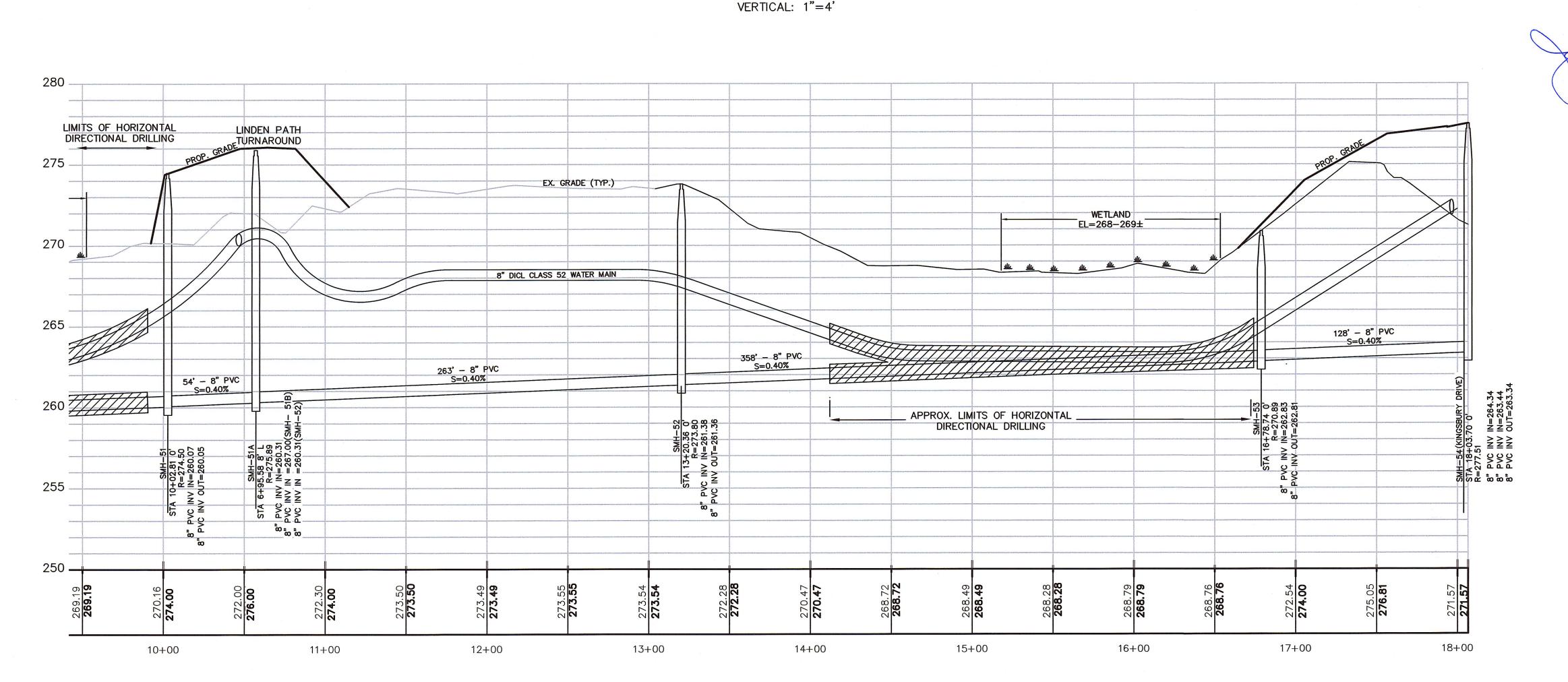
DRAWN BY: CJV CHECKED BY: JAP

SCALE: 1"=40' SHEET 50 OF 61

2765

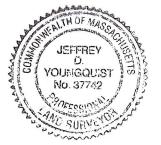


EAST-WEST UTILITY PROFILE
HORIZONTAL: 1"=40'



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TOWN CLERK-TOWN OF MEDWAY

JAMES A. PAVLIK CIVIL NJ. 36488

PROFILE
EAST-WEST UTILITY
CONNECTION

"TIMBER CREST

ESTATES
&
KINGSBURY VILLAGE"
IN
MEDWAY
MASSACHUSETTS



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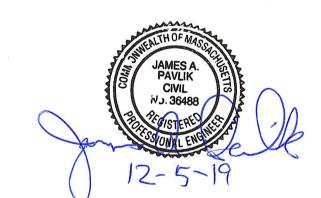
DATE: NOVEN	MBER 29, 2019
DRAWN BY: CJV	CHECKED BY: JAF
SCALE: 1"=40'	SHEET 51 OF 61
0' 40'	80' 12

SEWER CONNECTION TO BUTTERCUP LANE PROFILE HORIZONTAL: 1"=50' VERTICAL: 1"=5" -OHLSON CIRCLE (EXISTING UTILITIES NOT SHOWN) LOVERING STREET EX. GRADE SMH-5 STA 7+34.03 R=262.42 8" PVC INV 8" PVC INV 8" DICL CLASS 52 WATER MAIN 45 BEND (TYP.) 190' - 8" PVC S=0.40% 65" - 8" PVC S=0.60% PROP. 12'W 3-SIDED BOX CULVERT
OVER INTERMITTENT STREAM
EARTH BTM. EL.=267.7±
TOP CONC. EL.=270.9
(SEE DETAIL) 96' - 8" PVC S=0.60% CONNECT TO EXISTING 8" DICL WATER
MAIN IN OHLSON CIRCLE W/ 8"X8"X8" TEE
& SLEEVE & 3-WAY 8" GATE VALVE 81' - 8" PVC S=0.60% 1+00 2+00 3+004+00 5+00 6+007+00 8+00 9+0010 + 0011 + 0012+00 13 + 0014+00

SEE SHEET 31 FOR PLAN VIEW OF SEWER LINE STATIONING BETWEEN EXISTING SMH-1 AT BUTTERCUP LANE AND PROPOSED SMH-10

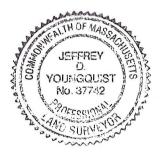
PROP. FINISH GRADE

EX. CL GRADE



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TOWN CLERK-TOWN OF MEDWAY

PROFILE
SEWER CONNECTION
TO
BUTTERCUP LANE

"TIMBER CREST
ESTATES"
IN
MEDWAY
MASSACHUSETTS

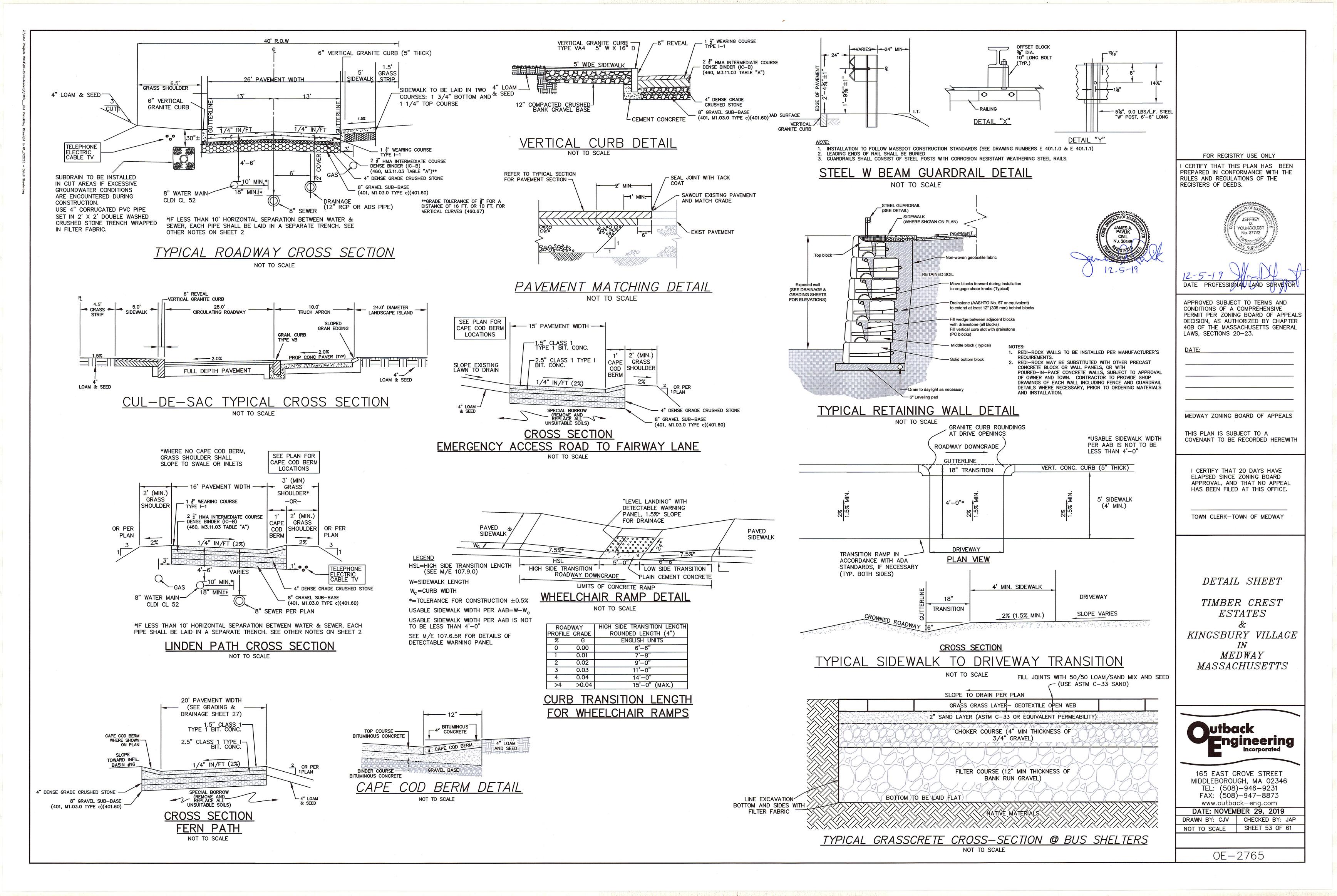
Putback Engineering Incorporated

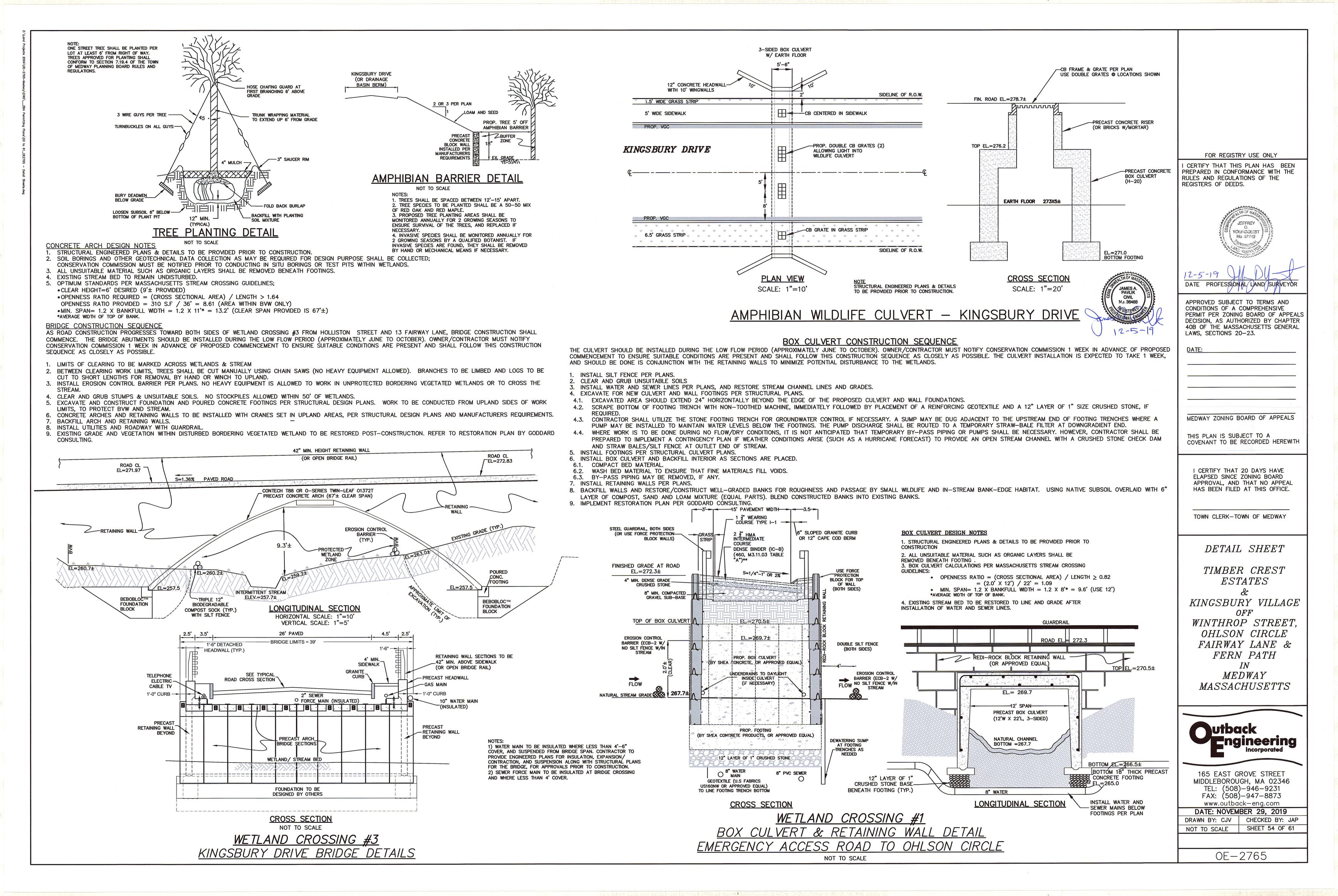
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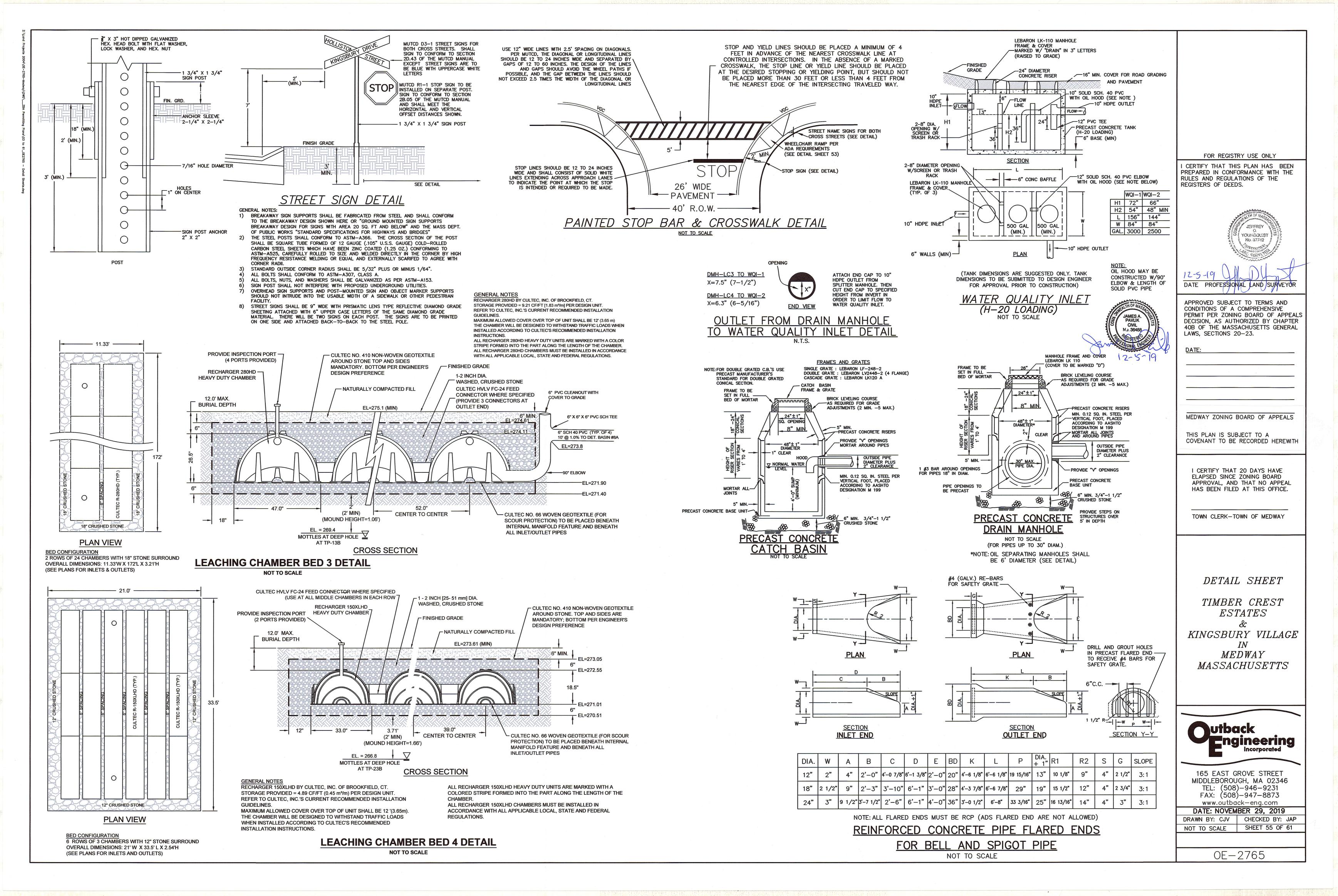
DATE: NOVEMBER 29, 2019

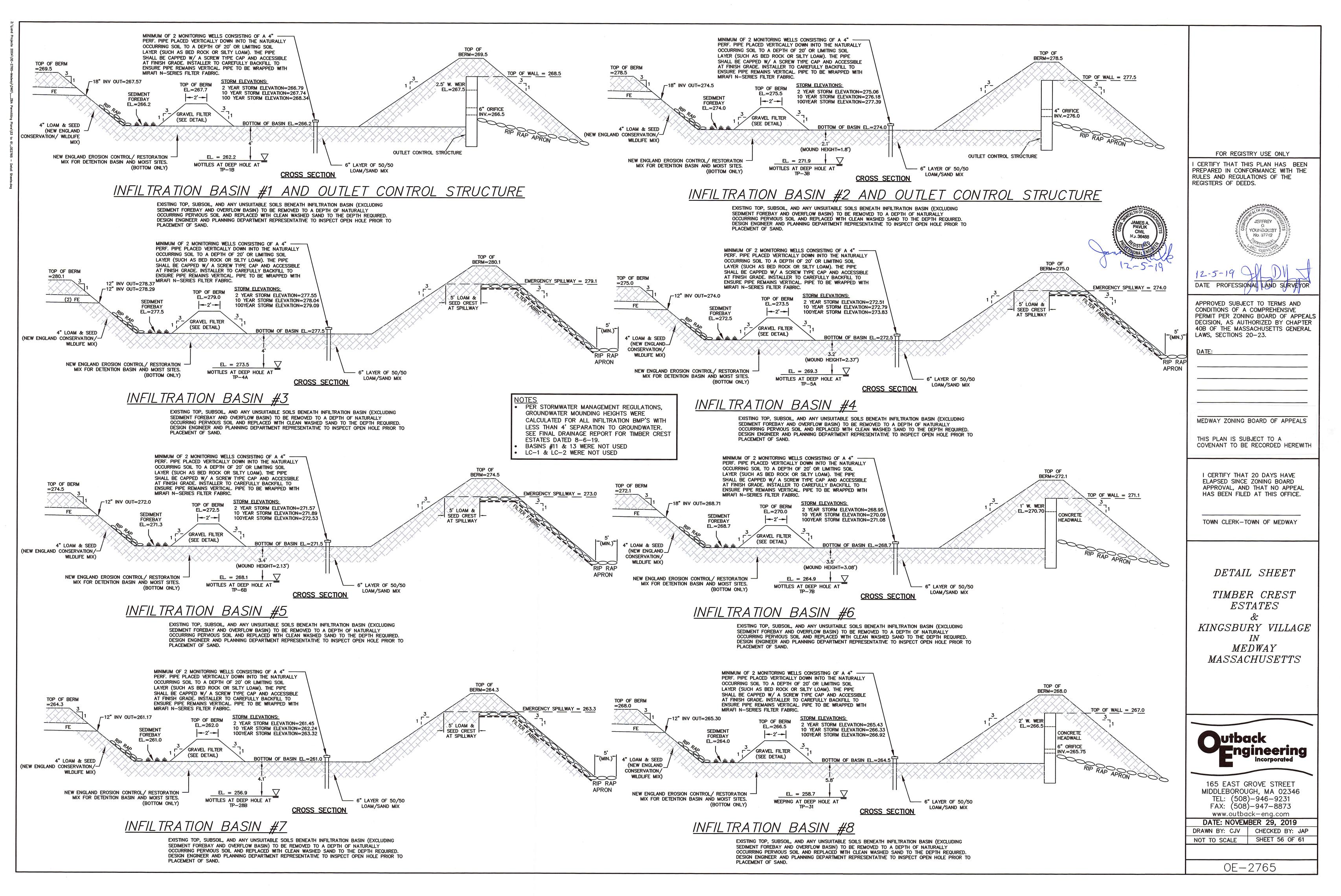
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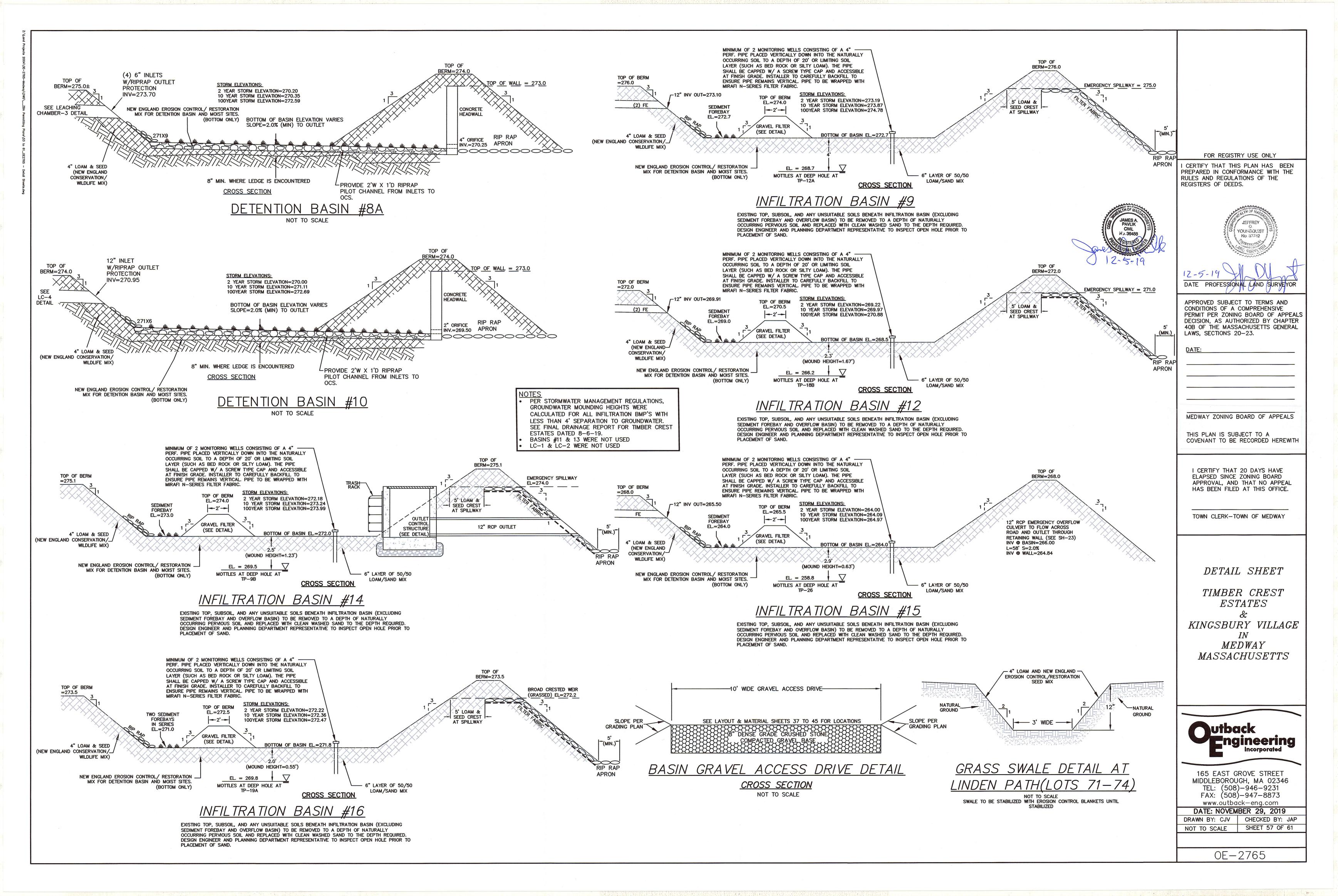
SCALE: AS SHOWN SHEET 52 OF 61

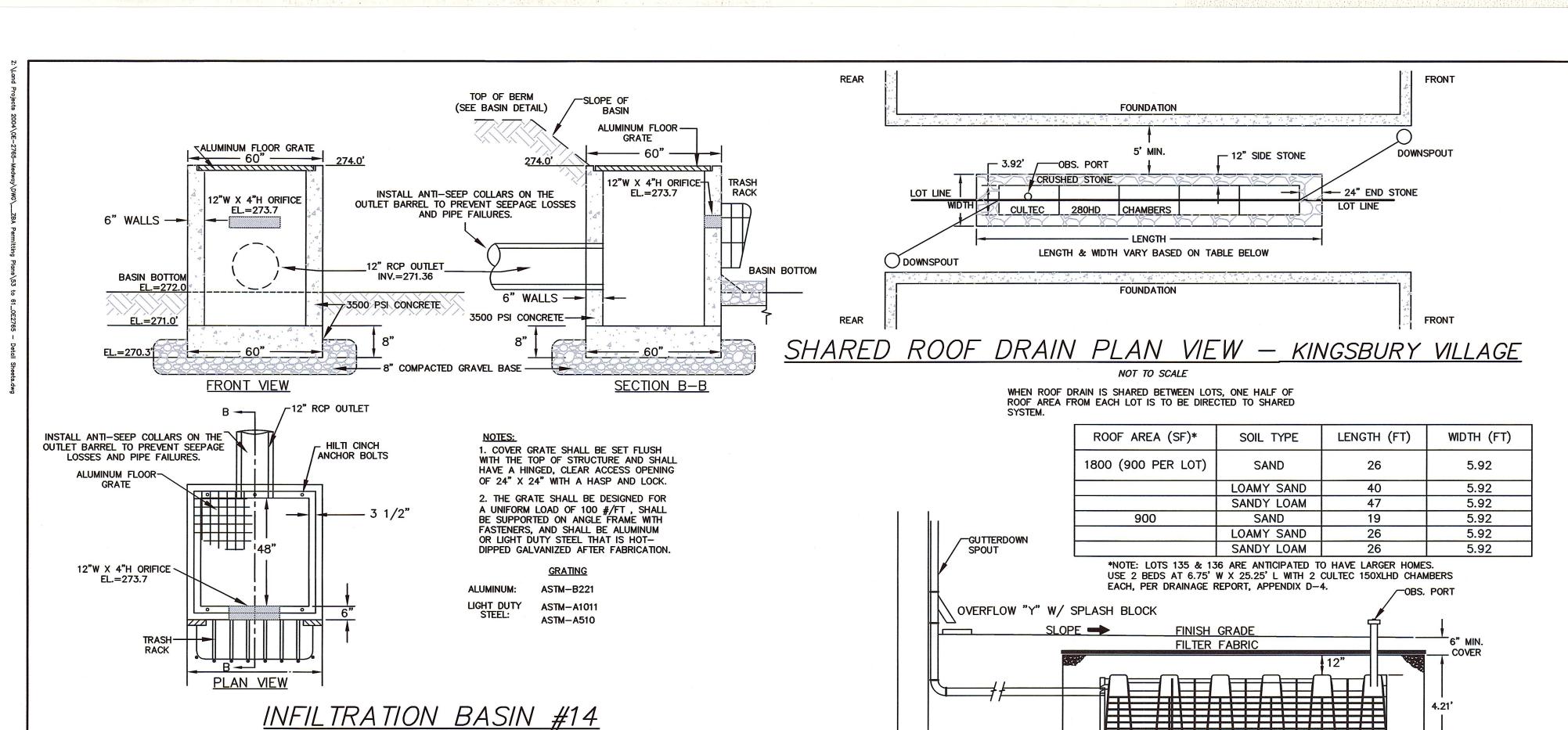




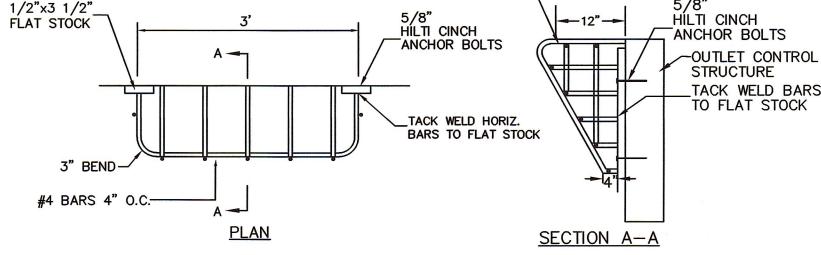






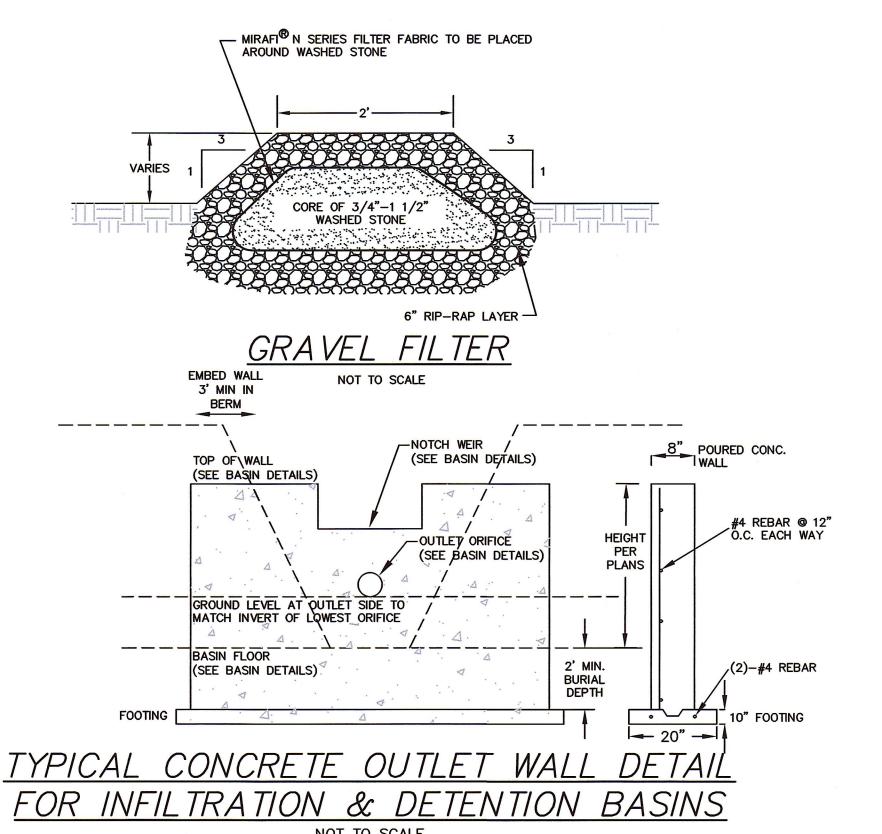


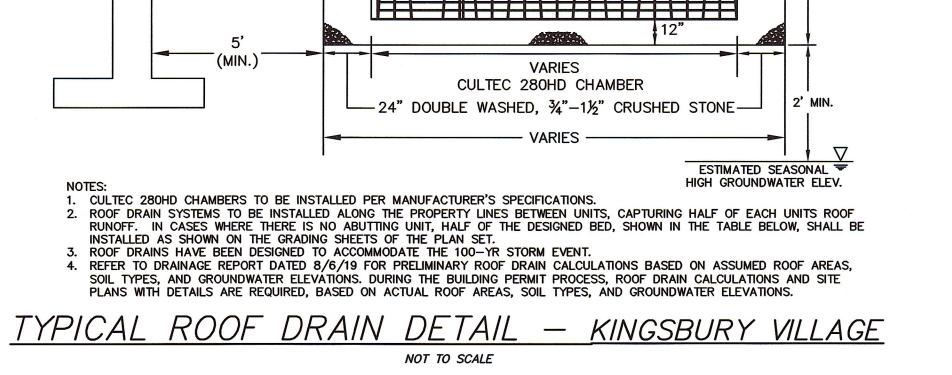
OUTLET CONTROL STRUCTURE DETAIL NOT TO SCALE HÍLTI CINCH HÍLTI CINCH ANCHOR BOLTS

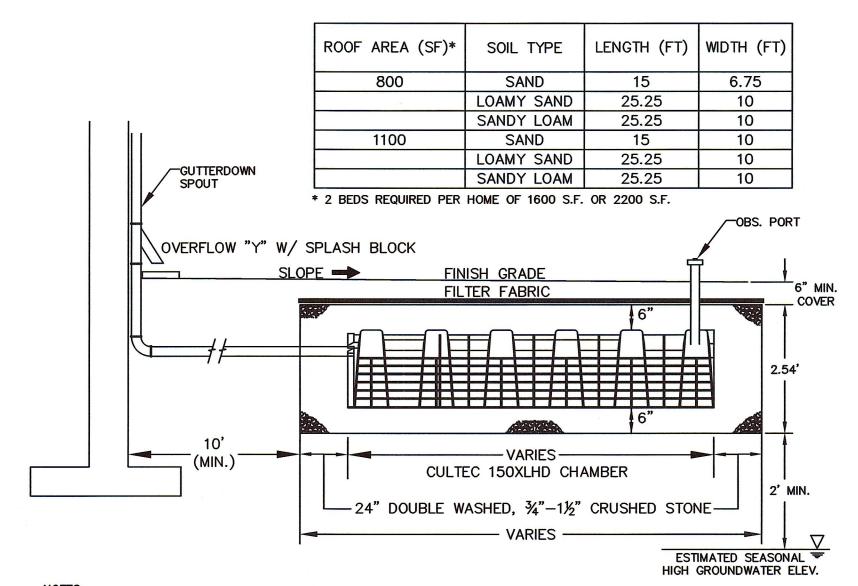


ALL COMPONENTS TO BE HOT DIPPED GALVANIZED OR STAINLESS STEEL 304

TRASH RACK



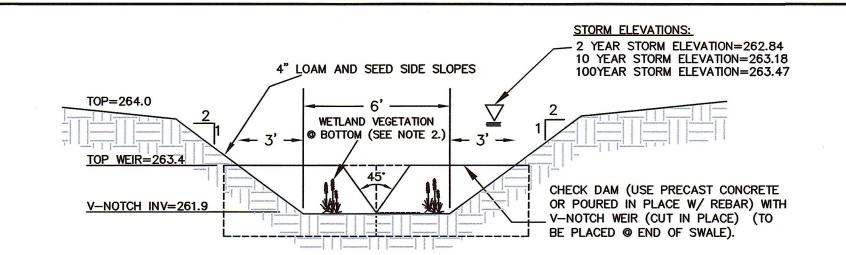




CULTEC 150XLHD CHAMBERS TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS. ROOF DRAIN SYSTEMS TO BE INSTALLED TO CAPTURE HALF OF EACH UNITS ROOF AREA. EXCEPT REAR ROOFS ON LOTS 19
AND 20 WHICH SHALL BE PIPED OR LAWN SLOPED TO BIO—RETENTION AREA #2 AND #3, RESPECTIVELY. BEDS MAY BE SPLIT UP INTO 2 OR MORE BEDS TO CAPTURE PORTIONS OF RÖOF AREAS.

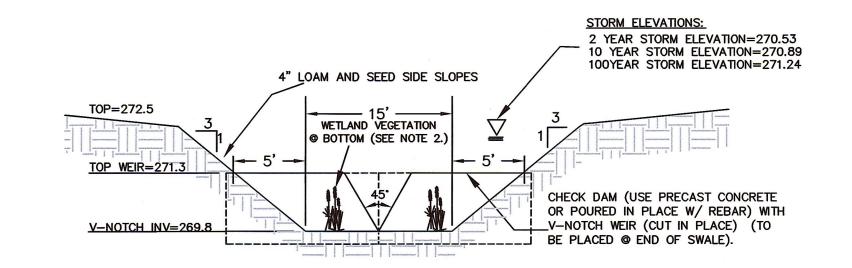
ROOF DRAINS HAVE BEEN DESIGNED TO ACCOMMODATE THE 100-YR STORM EVENT. REFER TO DRAINAGE REPORT DATED 8/6/19 FOR PRELIMINARY ROOF DRAIN CALCULATIONS BASED ON ASSUMED ROOF AREAS, SOIL TYPES, AND GROUNDWATER ELEVATIONS. DURING THE BUILDING PERMIT PROCESS, ROOF DRAIN CALCULATIONS AND SITE PLANS WITH DETAILS ARE REQUIRED, BASED ON ACTUAL ROOF AREAS, SOIL TYPES, AND GROUNDWATER ELEVATIONS.

TYPICAL ROOF DRAIN DETAIL - TIMBER CREST ESTATES



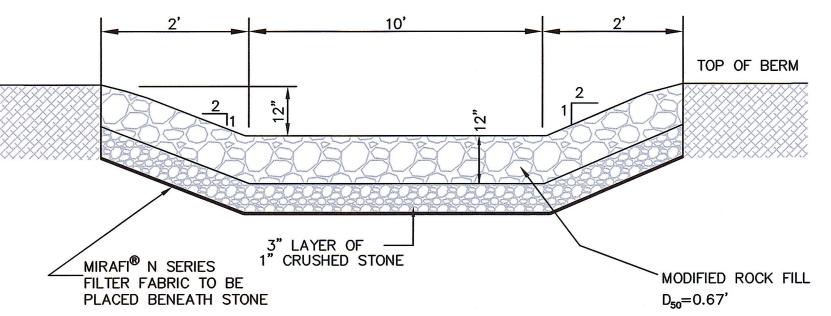
1. SWALE TO BE STABILIZED WITH EROSION CONTROL BLANKETS UNTIL STABILIZED, AS NECESSARY. BLANKETS MUST BIODEGRADE WITHIN ONE YEAR OF PLACEMENT. 2. BOTTOM OF SWALE TO BE LINED WITH EITHER 6 INCHES OF CLAY, OR SILT LOAM OR FINER MATERIAL TO PREVENT DRAWDOWN AND PROMOTE WETLAND VEGETATION GROWTH. BENTONITE OR POLYVINYL LINERS MAY ALSO BE USED (SEE BELOW) 3. USE COMMERCIALLY AVAILABLE WETLAND SEED MIX AT THE BOTTOM OF SWALES (NEW ENGLAND WETLAND PLANTS, INC, WETMIX" OR APPROVED EQUIVALENT) APPLIED AT A RATE OF 1 POUND OF SEED PER 2500 S.F. OF BOTTOM AREA.

WATER QUALITY SWALE #1 (WET SWALE) CROSS-SECTION



1. SWALE TO BE STABILIZED WITH EROSION CONTROL BLANKETS UNTIL STABILIZED, AS NECESSARY. BLANKETS MUST BIODEGRADE 2. BOTTOM OF SWALE TO BE LINED WITH EITHER 6 INCHES OF CLAY, OR SILT LOAM OR FINER MATERIAL TO PREVENT DRAWDOWN AND PROMOTE WETLAND VEGETATION GROWTH. BENTONITE OR POLYVINYL LINERS MAY ALSO BE USED (SEE BELOW) 3. USE COMMERCIALLY AVAILABLE WETLAND SEED MIX AT THE BOTTOM OF SWALES (NEW ENGLAND WETLAND PLANTS, INC. "WETMIX" OR APPROVED EQUIVALENT) APPLIED AT A RATE OF 1 POUND OF SEED PER 2500 S.F. OF BOTTOM AREA.

WATER QUALITY SWALE #2 (WET SWALE) CROSS-SECTION NOT TO SCALE



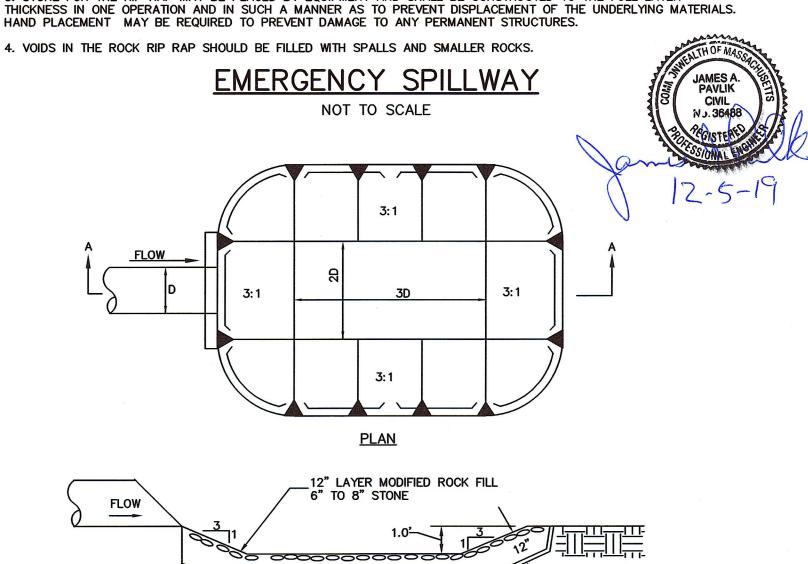
SPILLWAY NOTES:

1. A WELL GRADED MIXTURE OF ROCK SIZES SHALL BE USED FOR THE STONE. FIFTY PERCENT BY WEIGHT OF THE STONE MIXTURE SHALL BE SMALLER THEN THE MEDIAN SIZE STONE (d50). THE LARGEST STONE SIZE IN THE MIXTURE SHALL BE 1.5 TIMES THE d50 SIZE (12").

2. STONES FOR RIP RAP SHALL BE ANGULAR OR SUBANGULAR. THE STONES SHALL BE SHAPED SO THAT THE LEAST DIMENSION OF THE STONE FRAGMENT SHALL NOT BE LESS THAN ONE— THIRD OF THE GREATEST DIMENSION OF THE FRAGMENT. FLAT ROCKS SHALL NOT BE USED FOR RIP RAP.

3. STONE FOR THE RIP RAP MAY BE PLACED BY EQUIPMENT AND SHALL BE CONSTRUCTED TO THE FULL LAYER THICKNESS IN ONE OPERATION AND IN SUCH A MANNER AS TO PREVENT DISPLACEMENT OF THE UNDERLYING MATERIALS.

4. VOIDS IN THE ROCK RIP RAP SHOULD BE FILLED WITH SPALLS AND SMALLER ROCKS.



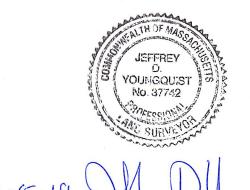
3"+ LAYER 1" CRUSHED STONE

RIP RAP OUTLET PROTECTION

SECTION A-A

FOR REGISTRY USE ONLY

CERTIFY THAT THIS PLAN HAS BEEN PREPARED IN CONFORMANCE WITH THE RULES AND REGULATIONS OF THE REGISTERS OF DEEDS.



APPROVED SUBJECT TO TERMS AND CONDITIONS OF A COMPREHENSIVE PERMIT PER ZONING BOARD OF APPEALS DECISION, AS AUTHORIZED BY CHAPTER 40B OF THE MASSACHUSETTS GENERAL

DATE PROFESSIONAL/LAND SURVEYOR

LAWS, SECTIONS 20-23.

MEDWAY ZONING BOARD OF APPEALS

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TOWN CLERK-TOWN OF MEDWAY

DETAIL SHEET

TIMBER CREST **ESTATES**

KINGSBURY VILLAGE MEDWAY

MASSACHUSETTS



165 EAST GROVE STREET MIDDLEBOROUGH, MA 02346 TEL: (508)-946-9231 FAX: (508)-947-8873 www.outback-eng.com

DATE: NOVEMBER 29, 2019 CHECKED BY: JAP DRAWN BY: CJV SHEET 58 OF 61 NOT TO SCALE

OE - 2765

CONSTRUCTION SPECIFICATIONS FOR BIORETENTION SYSTEMS

. MATERIAL SPECIFICATIONS

THE ALLOWABLE MATERIALS TO BE USED IN BIORETENTION AREAS ARE DETAILED IN TABLE 1.

PARAMETER	SPECIFICATION	NOTES
	SAND 80%	
PLANTING SOILS	SILT 15-20%	USDA SOIL TYPES LOAMY SANDS OR SANDY LOAM
	CLAY 0-5%	
	APPROVED	
COMPOST	NATURAL	•
	COMPOST	
	DOUBLE SHREDDED	
MULCH	HARDWOOD	AGED SIX (6) MONTHS MINIMUM
	NIDAE 440N	OR APPROVED EQUIVALENT
FILTER FABRIC	MIRAFI 140N	ON ALTROVED EQUIVALENT
	l .	1

2. PLANTING SOILS

THE SOIL SHOULD BE A UNIFORM MIX, FREE OF STONES, STUMPS, ROOTS OR OTHER SIMILAR OBJECTS LARGER THAN 2 INCHES. NO OTHER MATERIALS OR SUBSTANCES SHOULD BE MIXED OR DUMPED WITHIN THE BIORETENTION AREA THAT MAY BE HARMFUL TO PLANT GROWTH, OR PROVE A HINDRANCE TO THE PLANTING OR MAINTENANCE OPERATIONS. THE PLANTING SOIL SHOULD BE FREE OF NOXIOUS WEEDS.

THE BIORETENTION SYSTEM SHALL UTILIZE PLANTING SOIL HAVING A COMPOSITION AS SHOWN IN TABLE 1. THE BIORETENTION APPLICATIONS WITH A PLANTING SOIL DEPTH OF 12 INCHES, OR LESS, ADD 20% (BY VOLUME) OF WELL AGED (6-12 MONTHS), WELL AERATED, LEAF COMPOST (OR APPROVED EQUIVALENT) TO THE ABOVE PLANTING SOIL MIXTURE.

THE PLANTING SOIL SHOULD BE TESTED AND SHOULD MEET THE FOLLOWING CRITERIA:

PH RANGE 5.2-7.0 ORGANIC MATTER 1.5-4% MAGNESIUM 35 LB/AC PHOSPHORUS P205 75 LB/AC

POTASSIUM K20 85 LB/AC SOLUBLE SALTS NOT TO EXCEED 500 PPM

ALL BIORETENTION AREAS SHOULD HAVE A MINIMUM OF ONE TEST. EACH TEST SHOULD CONSIST OF BOTH THE STANDARD SOIL TEST FOR PH, PHOSPHORUS, AND POTASSIUM AND ADDITIONAL TESTS OF ORGANIC MATTER, AND SOLUBLE SALTS. A TEXTURAL ANALYSIS IS REQUIRED FROM THE SITE'S STOCKPILED TOPSOIL. IF TOPSOIL IS IMPORTED, THEN A TEXTURE ANALYSIS SHOULD BE PERFORMED FOR EACH LOCATION WHERE THE TOP SOIL IS EXCAVATED.

SINCE DIFFERENT LABS CALIBRATE THEIR EXISTING EQUIPMENT DIFFERENTLY, ALL TESTING RESULTS SHOULD COME FROM THE SAME TESTING FACILITY.

SHOULD THE PH FALL OUT OF THE ACCEPTABLE RANGE, IT MAY BE MODIFIED (HIGHER) WITH LIME OR (LOWER) WITH IRON SULFATE PLUS SULFUR.

MULCH LAYER SPECIFICATIONS

MULCH AROUND INDIVIDUAL PLANTS ONLY. SHREDDED HARDWOOD MULCH IS THE ONLY ACCEPTED MULCH. PINE MULCH AND WOOD CHIPS WILL FLOAT AND MOVE TO PERIMETER OF THE BIORETENTION AREA DURING A STORM EVENT AND ARE NOT ACCEPTABLE.

DOUBLE SHREDDED MULCH MUST BE WELL AGED (6-12 MONTHS) FOR ACCEPTANCE.

MIX APPROPRIATELY HALF OF THE SPECIFIED MULCH LAYER INTO THE PLANTING SOIL TO A DEPTH OF APPROXIMATELY 4 INCHES TO HELP FOSTER A HIGHLY ORGANIC SURFACE LAYER.

IT IS VERY IMPORTANT TO MINIMIZE COMPACTION OF BOTH THE BASE OF THE BIORETENTION AREA AND THE REQUIRED BACKFILL. WHEN POSSIBLE, USE EXCAVATION HOES TO REMOVE ORIGINAL SOIL IF BIORETENTION AREA IS EXCAVATED USING A LOADER, THE CONTRACTOR SHOULD USE WIDE TRACK OR MARSH TRACK EQUIPMENT, OR LIGHT EQUIPMENT WITH TURF TYPE TIRES. USE OF EQUIPMENT WITH NARROW TRACKS OR NARROW TIRES, RUBBER TIRES WITH LARGE LUGS, OR HIGH PRESSURE TIRES WILL CAUSE EXCESSIVE COMPACTION RESULTING IN REDUCED INFILTRATION RATES AND STORAGE VOLUMES AND IS NOT ACCEPTABLE. COMPACTION WILL SIGNIFICANTLY CONTRIBUTE TO DESIGN FAILURE.

COMPACTION CAN BE ALLEVIATED AT THE BASE OF THE BIORETENTION FACILITY BY USING A PRIMARY TILLING OPERATION SUCH AS A CHISEL PLOW, RIPPER, OR SUBSOILER. THESE TILLING OPERATIONS ARE TO REFRACTURE THE SOIL PROFILE THROUGH THE 12 INCH COMPACTION ZONE. SUBSTITUTE METHODS MUST BE APPROVED BY THE ENGINEER, ROTOTILLERS TYPICALLY DO NOT TILL DEEP ENOUGH TO REDUCE THE EFFECTS OF COMPACTION FROM HEAVY EQUIPMENT.

WHEN BACKFILLING THE BIORETENTION FACILITY, PLACE SOIL IN LIFTS OF 12 INCHES OR GREATER. DO NOT USE HEAVY EQUIPMENT WITHIN THE BIORETENTION AREA. HEAVY EQUIPMENT CAN BE USED AROUND THE PERIMETER OF THE BASIN TO SUPPLY SOILS AND SANDS. GRADE BIORETENTION MATERIALS WITH LIGHT EQUIPMENT SUCH AS A COMPACT LOADER OR A DOZER/LOADER WITH MARSH TRACKS.

5. PLANT INSTALLATION

THE PLANT ROOT BALL SHOULD BE PLANTED SO 1/8TH OF THE BALL IS ABOVE FINAL GRADE SURFACE. ROOT STOCK OF THE PLANT MATERIAL SHOULD BE KEPT MOIST DURING TRANSPORT AND ON-SITE STORAGE. THE DIAMETER OF THE PLANTING PIT SHOULD BE AT LEAST 6 INCHES LARGER THAN THE DIAMETER OF THE PLANTING BALL. SET AND MAINTAIN THE PLANT STRAIGHT DURING THE ENTIRE PLANTING PROCESS. THOROUGHLY WATER GROUND BED COVER AFTER INSTALLATION.

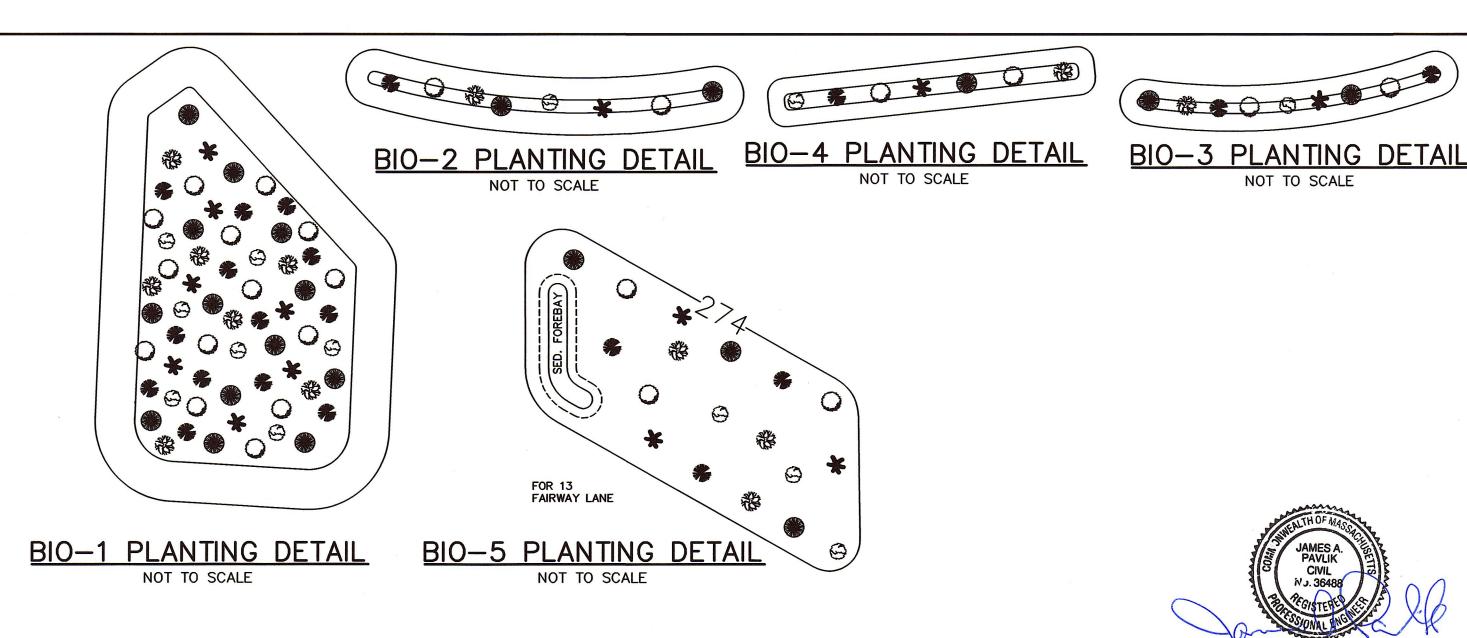
GRASSES AND LEGUME SEED SHOULD BE TILLED INTO THE SOIL TO A DEPTH OF AT LEAST ONE INCH. GRASS AND LEGUME PLUGS SHOULD BE PLANTED FOLLOWING THE NON-GRASS GROUND COVER PLANTING SPECIFICATIONS.

THE PLANTING SOIL SPECIFICATIONS PROVIDE ENOUGH ORGANIC MATERIAL TO ADEQUATELY SUPPLY NUTRIENTS FROM THE NATURAL CYCLING. THE PRIMARY FUNCTION OF THE BIORETENTION STRUCTURE IS TO IMPROVE WATER QUALITY. ADDING FERTILIZERS DEFEATS, OR AT A MINIMUM, IMPEDES THIS GOAL. ONLY ADD FERTILIZER IF COMPOST OR MULCH IS USED TO AMEND THE SOIL. ROTOTILL AREA FERTILIZER AT A RATE OF 2 LB/ 1,000 S.F..

6. MISCELLANEOUS

A. THE BIORETENTION FACILITY SHALL BE PROTECTED WITH SEDIMENT CONTROL DEVICES AS SHOWN IN THE EROSION & SEDIMENT CONTROL PLANS AND SHALL REMAIN OFFLINE AND INOPERATIONAL UNTIL ALL VEGETATION IS ESTABLISHED WITHIN THE BIORETENTION AREA AND THE CONTRIBUTING DRAINAGE AREA IS STABILIZED. B. THE PURPOSE OF THESE BIORETENTION AREAS IS TO PROVIDE SOME INCREASED RUNOFF WATER QUALITY AND INFILTRATION OF ROOF RUNOFF (FROM UNITS SHOWN) AND LIMITED LAWN AREAS; THESE RAIN GARDENS ARE DESIGNED TO OVERFLOW. THEY ARE NOT DESIGNED TO CONTROL RUNOFF RATES OR VOLUMES, OR FOR TSS REMOVAL CREDIT.

BIORETENTION AREA	BIORETENTION SURFACE ELEVATION	BIORETENTION SURFACE AREA	BOTTOM OF PLANTING SOIL ELEVATION	BOTTOM OF GRAVEL BASE/UNDERDRAIN ELEV.	TREES OR SHRUBS REQ'D (1/50 S.F.)
BIO-1	269.5	1998 S.F.	266.25	265.75	40
BIO-2	275.0	247 S.F.	271.75	271.25	5
BIO-3	275.0	266 S.F.	271.75	271.25	6
BIO-4	265.5	164 S.F.	262.25	261.75	4
BIO-5	273.0	421 S.F.	271.0	270.5	9

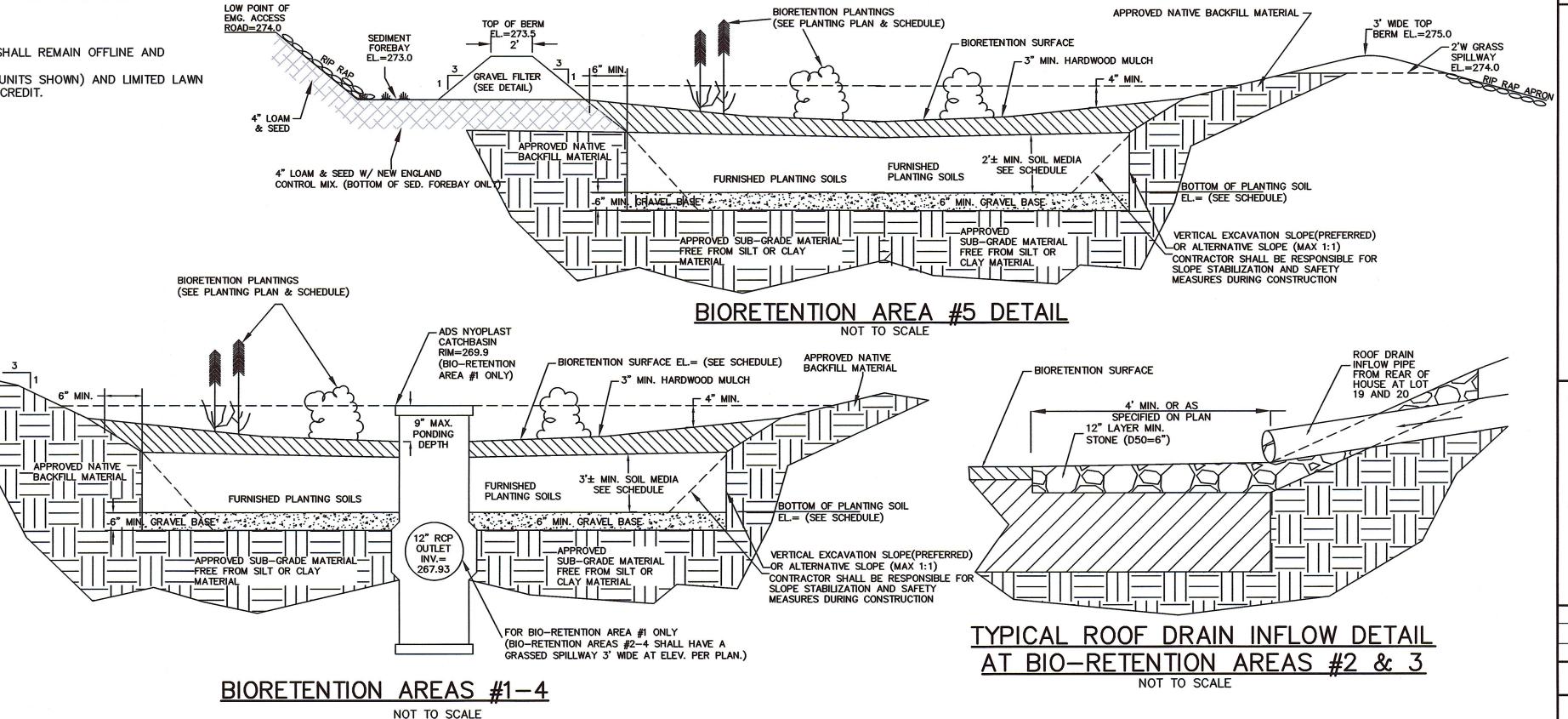


SHRUB SPEC	<u>IES</u>						QUANTI	ΓΥ	
PLAN SYMBOL	COMMON NAME (SCIENTIFIC NAME)	SIZE & FORM	IND. STATUS	SIZE @ PURCHASE	BIO-1	BIO-2	BIO-3	BIO-4	BIO-5
0	HIGHBUSH BLUEBERRY (VACCINIUM CORYMBOSUM)	MED-SIZED SHRUB (6-10')	FACW-	2-3 FEET	14	2	2	2	3
	SPICE BUSH (LINDERA BEZOIN)	MED-SIZED SHRUB (6-10')	FAC-	2-3 FEET	13	2	2	1	3
	SILKY DOGWOOD (CORMUS AMOMUM)	MED-SIZED SHRUB	FACW	2-3 FEET	13	1	2	1	3

<u>HERBACEOUS</u>	<u>SPECIES</u>					QUAN	NTITY		
PLAN SYMBOL	COMMON NAME (SCIENTIFIC NAME)	SIZE & FORM	IND. STATUS	SIZE @ PURCHASE	BIO-1	BIO-2	BIO-3	BIO-4	BIO-5
*	NEW YORK FERN (THELYPTERIS NOVEBORACENSIS)	FLOWER (1-3')	FAC		7 7	1	1	1	3
**	THREE-SQUARE BULRUSH (SCIRPUS PUNGENS)	FLOWER (1-3')	FACW+		7	1	1	1	3
9	BLACK-EYED SUSAN (RUDBECKIA HIRTA)	FLOWER (1-3')	FAC		7	1	1	1	3

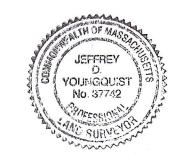
APPROVED NATIVE BACKFILL MATERIAL -

BIORETENTION PLANTINGS



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DATE PROFESSIONAL LAND SURVEYOR

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MEDWAY ZONING BOARD OF APPEALS

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TOWN CLERK-TOWN OF MEDWAY

DETAIL SHEET

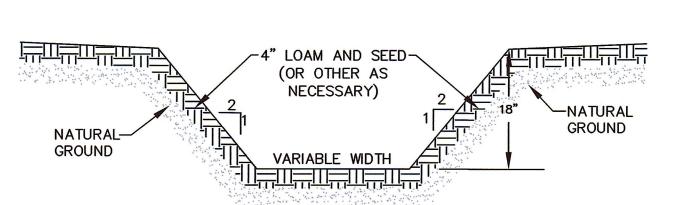
TIMBER CREST **ESTATES** KINGSBURY VILLAGE

MEDWAY*MASSACHUSETTS*

Dutback Engineering Incorporated

165 EAST GROVE STREET MIDDLEBOROUGH, MA 02346 TEL: (508)-946-9231 FAX: (508)-947-8873 www.outback-eng.com

DATE: NOVEMBER 29, 2019 DRAWN BY: CJV CHECKED BY: JAP SHEET 59 OF 61 NOT TO SCALE



TEMPORARY DIVERSION BERM/SWALE DETAIL

CONTRACTOR SHALL STABILIZE AS NECESSARY USING RIP RAP, CRUSHED STONE, EROSION CONTROL BLANKETS, AND/OR LOAM & SEED

CONSTRUCTION DEWATERING SUMP PIT DETAIL

CONSTRUCT A DEWATERING SUMP PIT IN ACCORDANCE WITH THE DETAIL

PROVIDED. TO PREVENT THE MIGRATION OF SILT AND SEDIMENT, THE CONTRACTOR

SHALL FIT THE DISCHARGE END OF THE PUMP HOSE WITH A COMPOSITE SOCK AND ALL FLOW SHALL BE DISCHARGED INTO A SEDIMENT CONTROL TRAP PRIOR

TO CONVEYANCE INTO THE STORM DRAIN SYSTEM. THE SEDIMENT CONTROL TRAP

SHALL BE CONSTRUCTED UTILIZING AN ENCLOSED AREA OF SILT FENCE AND/OR

FILTERMITT IN ACCORDANCE WITH THE DETAIL SHOWN. SEDIMENT TRAPS SHALL

DEWATERING PIT TO BE LOCATED OUTSIDE 100' BUFFER AND DISCHARGE, TO A

NOTE: IN LOCATIONS WHERE DEWATERING IS REQUIRED, THE CONTRACTOR SHALL

BE PLACED IN UPLAND AREAS ONLY AWAY FROM WETLANDS.

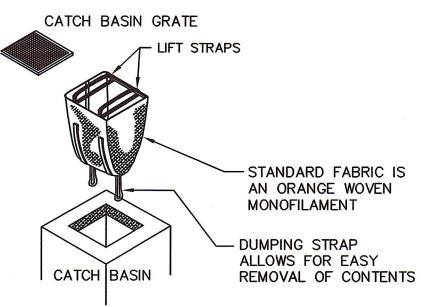
SEDIMENT TRAP OR TEMPORARY DEATERING PIT WHERE FEASIBLE

-PUMP DISCHARGE HOSE FITTED WITH W/F

CLEAN GRAVEL OR AASHTO M-43 #57 AGGREGATE FILL

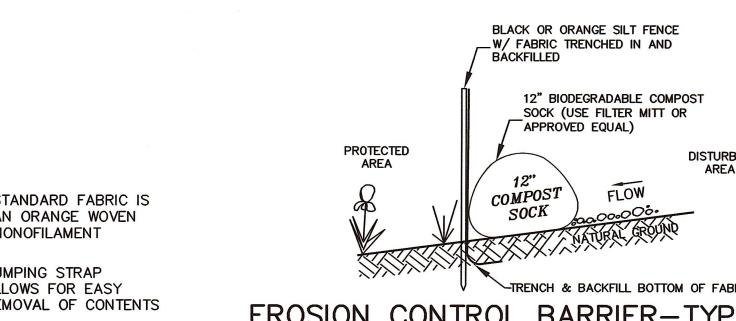
LSIDE SLOPE (VARIES)

- WATERTIGHT CAP OR PLATE



DANDY SACK® SILT SACK BY MIRAFI NOT TO SCALE

NOTE: SILT SACKS TO BE INSTALLED AT CATCH BASINS ONLY AFTER PAVED AREA IS CONSTRUCTED TO BASE COURSE PAVEMENT



EROSION CONTROL BARRIER-TYPE 1

REQUIRED AT 50'-100' OF BORDERING VEGETATED WETLANDS, SEE NOTE 1.

EROSION CONTROL BARRIER NOTES WHEN WORKING WITHIN 50'-100' OF A BORDERING VEGETATED WETLAND USE ECB-1.

BETWEEN 0'-50' OF A BORDERING VEGETATED WETLAND, USE ECB-2. (USE TRIPLE STACKED 12" BIODEGRADABLE COMPOST SOCK).

WITHIN WETLAND AT WETLAND CROSSINGS #1 AND #3 (EMG. ACCESS ROAD & KINGSBURY DRIVE BRIDGE), AND AT THE BVW WETLAND REPLICATION AREA USE ECB-2, EXCEPT, NO SILT FENCE SHALL BE USED. SET TRIPLE

COMPOST SOCKS ONLY. IF WORKING WITHIN 10 FEET TO BORDERING VEGETATED WETLAND RESOURCE AREA, 7-DAY SITE STABILIZATION IS REQUIRED IN ADDITION TO ECB-2.

• UPON COMPLETION OF WORK, OR IF SITE IS TO HAVE WORK STOPPAGE FOR MORE THAN 7-DAYS, THE SITE MUST BE STABILIZED TEMPORARILY WITH JUTE MESH, MULCH, HYDROSEED, OR PERMANENT GROUND COVER PER PLANS.

OWN OF MEDWAY

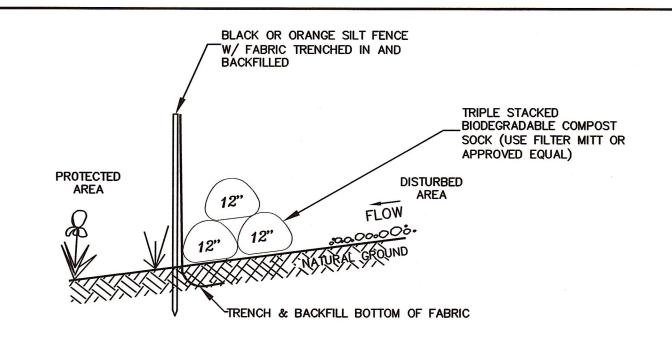
SENSITIVE WETLAND

RESOURCE AREA

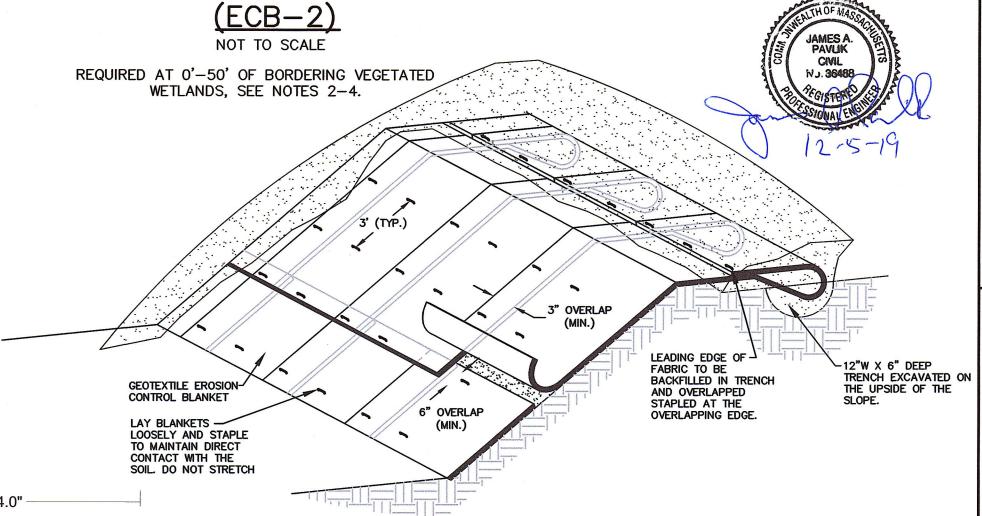
NO DUMPING

THIS AREA PROTECTS YOUR NEIGHBORHOOD FROM FLOODING AND GROUNDWATER POLLUTION

SIGN

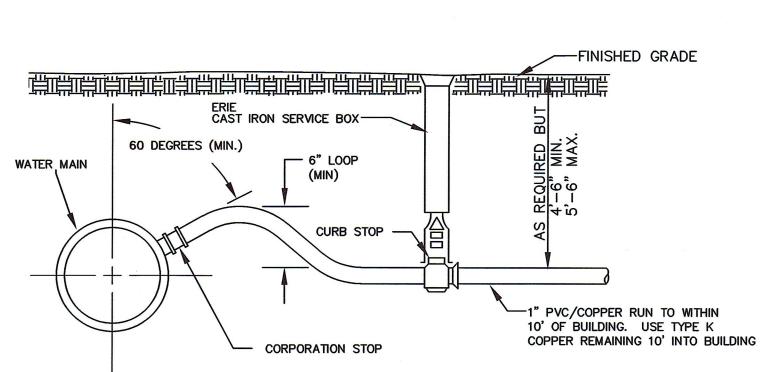


EROSION CONTROL BARRIER-TYPE 2



GEOTEXTILE EROSION

NOTE
USE ON 2:1 SLOPES AND WHERE NECESSARY TO PREVENT EROSION.



NOTE: SADDLES ARE REQUIRED FOR TAPS LARGER THAN 1 INCH.

TYPICAL PERMANENT WATER SERVICE CONNECTION

1'-6" ROTATE HYDRANT AS REQUIRED 2" MIN. 6" MAX.─┐ ADJUST HYDRANT TO GRADE AS REQUIRED ---VALVE BOX (ADJUSTABLE)

outback **Engineering** Incorporated

FOR REGISTRY USE ONLY

I CERTIFY THAT THIS PLAN HAS BEEN

PREPARED IN CONFORMANCE WITH THE

DATE PROFESSIONAL LAND SURVEYOR

APPROVED SUBJECT TO TERMS AND

PERMIT PER ZONING BOARD OF APPEALS

DECISION, AS AUTHORIZED BY CHAPTER

40B OF THE MASSACHUSETTS GENERAL

MEDWAY ZONING BOARD OF APPEALS

COVENANT TO BE RECORDED HEREWITH

I CERTIFY THAT 20 DAYS HAVE

ELAPSED SINCE ZONING BOARD

APPROVAL, AND THAT NO APPEAL

HAS BEEN FILED AT THIS OFFICE.

TOWN CLERK-TOWN OF MEDWAY

DETAIL SHEET

TIMBER CREST

ESTATES

KINGSBURY VILLAGE

MEDWAY

MASSACHUSETTS

THIS PLAN IS SUBJECT TO A

CONDITIONS OF A COMPREHENSIVE

LAWS, SECTIONS 20-23.

YOUNGQUIST

No. 37782

RULES AND REGULATIONS OF THE

REGISTERS OF DEEDS.

165 EAST GROVE STREET MIDDLEBOROUGH, MA 02346 TEL: (508)-946-9231 FAX: (508)-947-8873 www.outback-eng.com

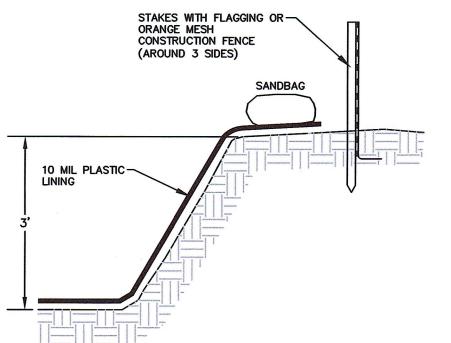
DATE: NOVEMBER 29, 2019 DRAWN BY: CJV | CHECKED BY: JAP NOT TO SCALE SHEET 60 OF 61

2" STONE PER MA DPW-6" MINIMUM THICKNESS / W/ FILTER FABRIC BENEATH

TEMPORARY SITE CONSTRUCT

ENTRANCE PAD

N.T.S.

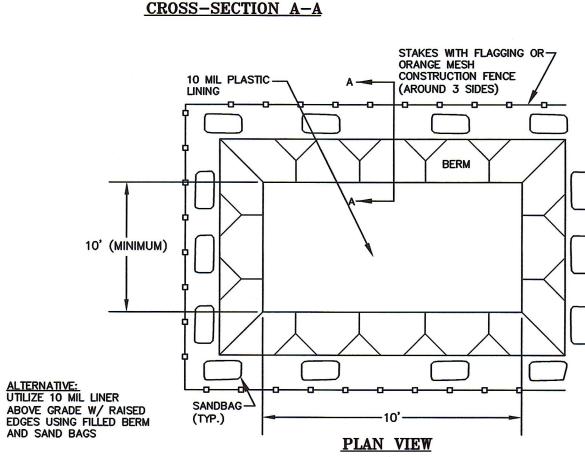


12"-18"

EXISTING GROUND -

STANDPIPE WRAPPED IN 1/2 INCH HARDWARE CLOTH ÁND GEOTEXTILE CLASS E

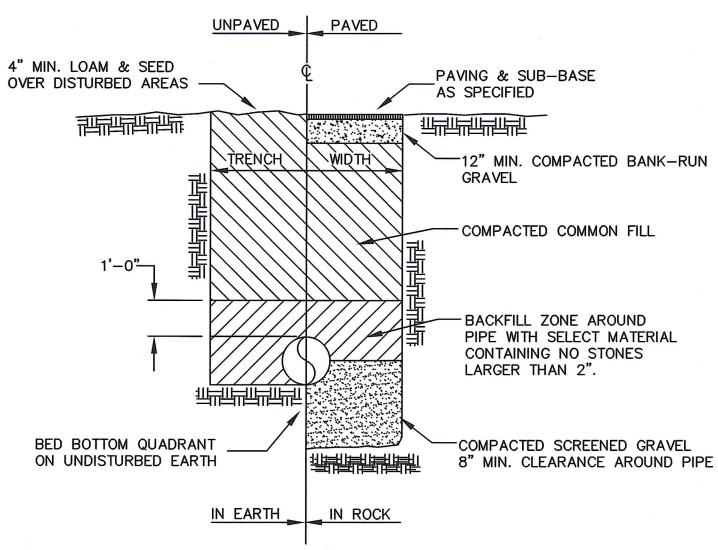
12"-36" DIA. PERFORATED CORRUGATED METAL OR



BELOW GRADE CONCRETE WASHOUT PIT NOT TO SCALE

1). TRENCH SHALL BE A MIN. OF 3' IN WIDTH PER TOWN OF MEDWAY STANDARDS. TRENCH SHALL BE 5' MAX. DEPTH FOR STEEP SIDED SECTION TRENCH SHALL BE PROVIDED WITH 6" MIN. CRUSHED STONE BEDDING PER TOWN OF MEDWAY.

NOT TO SCALE



NOT TO SCALE - FINISHED GRADE VALVE BOX (ADJUSTABLE) -GATE VALVE 3000 PSI CONCRETE SUPPORT UNDISTURBED MATERIAL

NOT TO SCALE POLLUTION PREVENTION & STORMWATER SIGN DETAIL "TOWN OF MEDWAY STANDARD -HYDRANT AND THREAD" FINISHED GRADE-AS REQUIRED BUT NOT LESS THAN 4'-6" MIN. 1/2 C.Y. OF 3/4" CRUSHED STONE WITHIN A PIT 2'-6" MIN. DIAM. TO AT LEAST 6" ABOVE -6" GATE VALVE 🞇 HYDRANT DRAIN RING. 2 TIE RODS¬ 6" C.L.D.I. PIPE -3000 PSI CONCRETE THRUST BLOCK AGAINST UNDISTURBED MATERIAL -ANCHORING TEE

3000 PSI CONCRETE — THRUST BLOCK AGAINST UNDISTURBED EARTH

TOWN OF MEDWAY

STORMWATER

RESOURCE AREA

NO DUMPING

THIS AREA PROTECTS YOUR NEIGHBORHOOD FROM FLOODING AND GROUNDWATER POLLUTION

KEEP IT CLEAN AND OPEN. ALLOW TURAL VEGETATION TO STOP EROS

NOT TO SCALE

TYPICAL GATE VALVE NOT TO SCALE

WATER TRENCH

(RESTRAINED M.J.) FLAT STONE OR CONCRETE BLOCK SET ON UNDISTURBED SOIL. TYPICAL HYDRANT & VALVE

BUOYANCY CALCULATIONS (LIFT STATION 1)

MAX. GROUNDWATER ELEV. = 268.0 (ASSUMED AT EXISTING GROUND SURFACE) PROPOSED CONCRETE SLAB BOTTOM ELEV.= 256.0 BUOYANCY FORCE ON CONCRETE SLAB (USE 8' X 8'): VOLUME DISPLACED = $64 \text{ FT}^2 \text{ X } (268.0 - 256.0)$

= 768 C.F.WT. OF DISPLACED WATER = $768 \text{ C.F. } \times 62.4 \text{ \#/C.F.}$

= 47923 # ↑ WEIGHT OF EMPTY TANK:

WT. OF EMPTY TANK = $750\pm \#$

PROVIDE 8' X 8'

CONCRETE SLAB BASE & COLLAR FOR BALLAST >

(#5 REBAR, 12" O.C. E.W.

PROVIDE 18" DEEP CONCRETE

COLLAR W/ #5 REBAR ALL

F.G.=269.5 (LIFT STATION #

F.G.=266.5 (LIFT STATION #3)

2" INV. OUT=265.0 (LIFT STATION #1) _____ 2" INV. OUT=260.5 (LIFT STATION #3) ____

72"ø X 140" DEEP

2" DISCHARGE

2" CHECK VALVE

EL.=258.0 (LIFT STATION ;

EL.=255.0 (LIFT STATION

PROVIDE 8' X 8' X 2' D

(#5 REBAR, 12" O.C. E.W.

CONCRETE SLAB BASE

FOR BALLAST

FIBERGLASS CHAMBER

W/ ISOLATION VALVES

AND CHECK VALVES W/

8" INV. IN=263.55 (LIFT STATION #

8" INV. IN=260.39 (LIFT STATION #3

WEIGHT OF CONCRETE BALLAST BASE & COLLAR: VOLUME = 2' DEEP X 64.0 $FT^2 + 35.7 FT^2 \times 1.5' = 181.55 C.F.$ WEIGHT = 181.55 C.F. X 150 $\#/\text{C.F.} = 27233 \# \downarrow$

WEIGHT OF SOIL ATOP BALLAST SLAB: VOLUME = 10.5' DEEP X 35.7 FT² = 375 C.F. WEIGHT = 375 C.F. X 120 $\#/\text{C.F.} = 44982 \# \downarrow$

4a. BUOYANCY FORCE OF SOIL ATOP SLAB: VOLUME = (268-258) DEEP X 35.7 FT² = 411 C.F. WEIGHT = 411 C.F. X 62.4 #/C.F. = 22276 # \uparrow SUM OF FORCES: $44982 \downarrow -22276 \uparrow = 22706 \downarrow \#$

F.S. = $(750 \# + 27233 \# + 22706 \#) \div 47923 \# = 1.1 FACTOR OF SAFETY$ WT. OF CONCRETE BALLAST, COLLAR, TANK + SOIL IS GREATER THAN WT. OF DISPLACED WATER

UNICHECK VALVE

LIFT STATIONS #1

BUOYANCY CALCULATIONS (LIFT STATION 2)

MAX. GROUNDWATER ELEV. = 270.7 (ASSUMED AT EXISTING GROUND SURFACE) PROPOSED TANK BOTTOM ELEV.= 250.0 BUOYANCY FORCE ON EMPTY TANK: VOLUME DISPLACED = $81.25 \text{ FT}^2 \text{ X } (270.7 - 250.0)$

= 1682 C.F.WT. OF DISPLACED WATER = $1682 \text{ C.F. } \times 62.4 \text{ #/C.F.}$ = 104949 # 1

WEIGHT OF EMPTY TANK: WT. OF EMPTY TANK = 69000 #

JUSE TIE DOWN STRAPS PER

MANUFACTURER (TYP.)

CONCRETE

A=35.7 FT²

GW EL.=268.0 (LIFT STATION #1)

GW EL.=262.0 (LIFT STATION #3)

ASSUME @ EX. GROUND SURFACE)

DUPLEX ZOELLER

SS LIFT CABLES,

& SS RAIL GUIDES

PUMPS, REVERSIBLE :

HP EXPLOSION PROOF

WITH FLOAT CONTROLS

EL.=256.0 (LIFT STATION #1)

EL.=253.0 (LIFT STATION #3)

PUMP CORDS NOT SHOWN FOR CLARITY.

_6" MIN. 3/4"-1 1/2"

CRUSHED STONE

NOTES FOR LIFT STATION 1-3:

(ASSUME @ EX. GROUND SURFACE)

-ALUMINUM HATCH

TOP OF STATION #1 EL.=270.0

TOP OF STATION #3 EL.=267.0

WEIGHT OF SOIL ATOP BASE: VOLUME = 24.3' DEEP X $29 \text{ FT}^2 = 705 \text{ C.F.}$ WEIGHT = 705 C.F. X 120 $\#/\text{C.F.} = 84564 \# \downarrow$

3a. BUOYANCY FORCE OF SOIL ATOP SLAB: VOLUME = (270.7-250.7)' DEEP X 29 FT² = 580 C.F. WEIGHT = 580 C.F. X 62.4 $\#/\text{C.F.} = 36192 \# \uparrow$ SUM OF FORCES: $84564 \downarrow -36192 \uparrow = 48372 \downarrow \#$

4. SUM OF FORCES: F.S. = $(69000 \# + 48372 \#) \div 104949 \# = 1.1 FACTOR OF SAFETY$ WT. OF TANK + SOIL IS GREATER THAN WT. OF DISPLACED WATER ∴ OK

BUOYANCY CALCULATIONS (LIFT STATION 3)

MAX. GROUNDWATER ELEV. = 262.0 (ASSUMED AT EXISTING GROUND SURFACE)

PROPOSED CONCRETE SLAB BOTTOM ELEV .= 253.0 1. BUOYANCY FORCE ON CONCRETE SLAB (USE 8' X 8'): VOLUME DISPLACED = $64.0 \text{ FT}^2 \text{ X} (262.0 - 253.0)$ = 576 C.f.

WT. OF DISPLACED WATER = 576 C.F. X 62.4 #/C.F. =35942 # 1

WEIGHT OF EMPTY TANK: WT. OF EMPTY TANK = $750\pm$ #

WEIGHT OF CONCRETE BALLAST BASE & COLLAR: VOLUME = 2' DEEP X 64.0 FT^2 + 35.7 FT^2 X 1.5' = 181.55 C.F. WEIGHT = 181.55 C.F. X 150 $\#/\text{C.F.} = 27233 \# \downarrow$

WEIGHT OF SOIL ATOP SLAB: VOLUME = 10.5' DEEP X 35.7 FT² = 375 C.F. WEIGHT = 375 C.F. X 120 $\#/\text{C.F.} = 44982 \# \downarrow$

4a. BUOYANCY FORCE OF SOIL ATOP SLAB: VOLUME = (262-255)' DEEP X 35.7 FT² = 250 C.F. WEIGHT = 250 C.F. X 62.4 #/C.F. = 15593 # \uparrow SUM OF FORCES: $44982 \downarrow -15593 \uparrow = 29388 \downarrow \#$

5. SUM OF FORCES: F.S. = $(750 \# + 27233 \# + 29388 \#) \div 35942 \# = 1.6 FACTOR OF SAFETY$ WT. OF CONCRETE BALLAST, TANK, + SOIL IS GREATER THAN WT. OF DISPLACED WATER

BUOYANCY CALCULATIONS (SMH 51)

MAX. GROUNDWATER ELEV. = 268.6 (BASED ON MOTTLES @ 18" IN TP-9B) PROPOSED SMH 51 BOTTOM ELEV.= 259.3

1. BUOYANCY FORCE ON STRUCTURE: VOLUME DISPLACED = $18.3 \text{ FT}^2 \text{ X} (268.6 - 259.3)$ = 170.19 C.F. WT. OF DISPLACED WATER = 170.19 C.F. X 62.4 #/C.F.

=10619.9 # WEIGHT OF SMH STRUCTURE: VOLUME = 15.2' DEEP X 5.76 FT² = 87.6 C.F. WEIGHT = 87.6 C.F. X 150 #/C.F. = 13,132.8 #

SUM OF FORCES: F.S. = $(13,132.8 \ \#) \div 10,619.9 \ \# = 1.24 \ FACTOR \ OF \ SAFETY$ WT. OF SMH IS GREATER THAN WT. OF DISPLACED WATER ∴ OK

BUOYANCY CALCULATIONS (SMH 54)

MAX. GROUNDWATER ELEV. = 269.2 (BASED ON MOTTLES @ 24" IN TP-12A) PROPOSED SMH 54 BOTTOM ELEV.= 262.9 BUOYANCY FORCE ON STRUCTURE:

VOLUME DISPLACED = $18.3 \text{ FT}^2 \text{ X } (269.2 - 262.9)$ = 115.29 C.F.

WT. OF DISPLACED WATER = 115.29 C.F. X 62.4 #/C.F. =7294.1 # ^

WEIGHT OF SMH STRUCTURE: VOLUME = 14.6' DEEP X 5.76 FT² = 84.1 C.F. WEIGHT = 84.1 C.F. X 150 #/C.F. = 12,614.4 #

OMIT TOPSOIL WHERE NO GRASS

OCCURS. USE 6" SELECT GRAVEL

6" GRAVEL

3/4" CRUSHED-

SHEETING AS REQUIRED

STONE

6" TOPSOIL

SUM OF FORCES: F.S. = $(12,614.4 \ \#) \div 7294.1 \ \# = 1.73 \ FACTOR \ OF \ SAFETY$ WT. OF SMH IS GREATER THAN WT. OF DISPLACED WATER

TYPICAL BUOYANCY CALCULATION FOR SEWER MANHOLES

SECTION UNDER GRASS | SECTION UNDER PAVEMENT

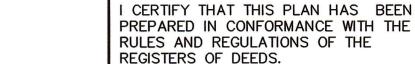
5' AVERAGE TRENCH WIDTH

MIN. V

. PAY WIDTH EQUALS 5 FEET

REPORTED

MIN.



THESE CALCS ARE REPRESENTATIVE

OF THE WORST CASE SCENARIOS OF

PROJECT SITE. ALL SMH BUOYANCY

CONDITIONS WERE NOT CALCULATED.

CIVIL VJ. 36488

SMH TO BE INSTALLED AT THE



FOR REGISTRY USE ONLY

DATE PROFESSIONAL LAND SURMEYOR

APPROVED SUBJECT TO TERMS AND CONDITIONS OF A COMPREHENSIVE PERMIT PER ZONING BOARD OF APPEALS DECISION, AS AUTHORIZED BY CHAPTER 40B OF THE MASSACHUSETTS GENERAL LAWS, SECTIONS 20-23.

MEDWAY ZONING BOARD OF APPEALS

THIS PLAN IS SUBJECT TO A COVENANT TO BE RECORDED HEREWITH

I CERTIFY THAT 20 DAYS HAVE ELAPSED SINCE ZONING BOARD APPROVAL, AND THAT NO APPEAL HAS BEEN FILED AT THIS OFFICE.

TOWN CLERK-TOWN OF MEDWAY

DETAIL SHEET

TIMBER CREST

ESTATES KINGSBURY VILLAGE

MEDWAY *MASSACHUSETTS*

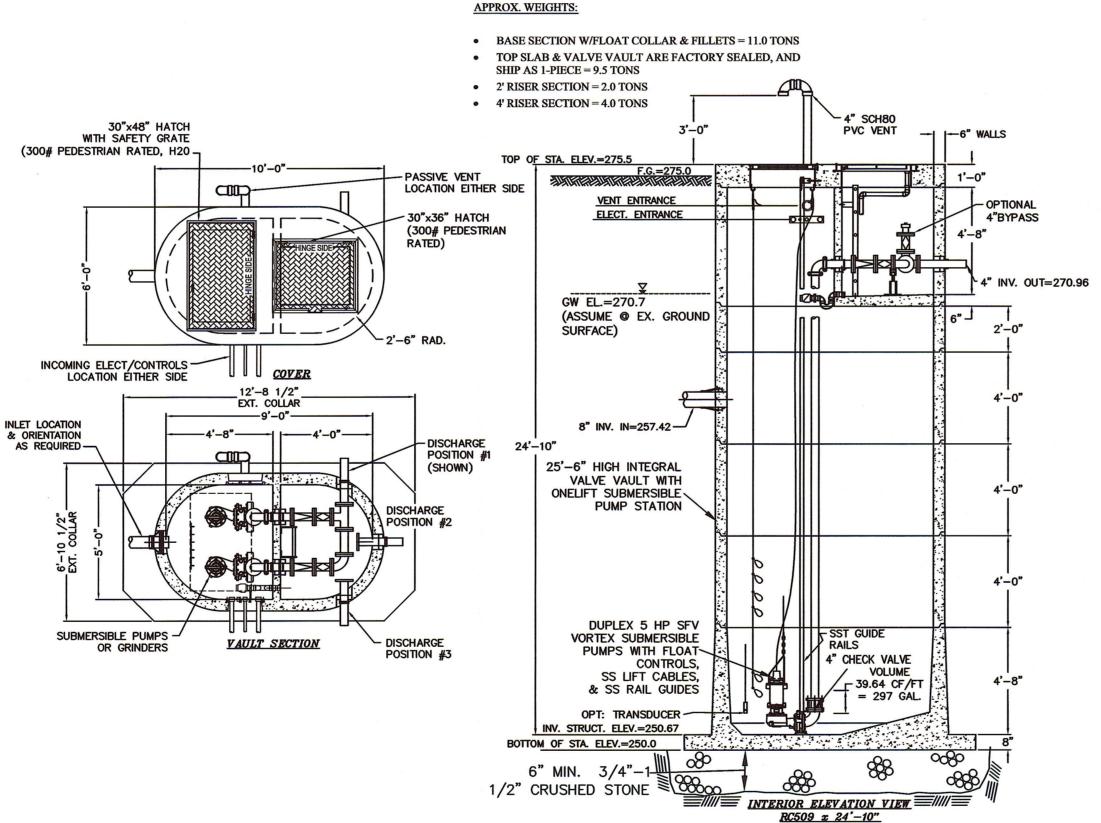
Tutback **Engineering** Incorporated

165 EAST GROVE STREET MIDDLEBOROUGH, MA 02346 TEL: (508)-946-9231 FAX: (508)-947-8873

DATE: NOVEMBER 29, 2019 DRAWN BY: CJV | CHECKED BY: JAP NOT TO SCALE SHEET 61 OF 61

0E - 2765

LIFT STATION BUOYANCY CALCULATIONS



2. CONTRACTOR TO PROVIDE ALL EQUIPMENT, MATERIALS & LABOR NECESSARY FOR COMPLETE INSTALLATION OF LIFT STATIONS (SEE HAYES PUMP, INC. BUDGET QUOTATION LETTERS DATED NOVEMBER 12, 2018 FOR SPECIFICATIONS.), INCLUDING ELECTRIC CONTROL PANELS AND BACKUP GENERATORS. CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR REVIEW BY DESIGN ENGINEER & MEDWAY DPW PRIOR TO ORDERING AND INSTALLATION AND AS-BUILT DRAWINGS PRIOR TO PLACING STATIONS IN SERVICE.

3. PRIOR TO RELEASE OF ANY BUILDING PERMITS FOR INDIVIDUAL LOTS, CONTRACTOR SHALL PROVIDE THE OWNER & TOWN DPW WITH A CLEAR WELL TEST REPORT AND AS-BUILT DRAWINGS FOR ALL PUMP STATIONS NEEDED TO SERVE PROPOSED BUILDING LOTS. OWNER SHALL PROVIDE A COPY OF A CONTRACT FOR EMERGENCY AND ROUTINE MAINTENANCE SERVICES DESIGNATING WHO IS RESPONSIBLE FOR SUCH SERVICES AND RELATED CONTACT INFORMATION. (REFER TO SEWER LIFT STATIONS SPEC SHEETS & STANDARD OPERATING GUIDELINE TIMBER CREST ESTATES & KINGSBURY VILLAGE DATED NOVEMBER 29, 2019.)

LIFT Station # & Avg Flow(GPD) Peak flow (5X) 8" Inv. In to Inv. Out (of Connecting (GPD) # of homes (170 GPD/home) GPH (peak hour) GPM (peak hour) lRim El. FM length linv. El. Location Lift station wet well) 263.55 273.40 #1 / Rosewood Dr. 11 1870 9350 390 270 (2") 265 *204' 12070 2515 276 (4") 270.96 #2/ Infil. Bas. 4 71 60350 42 257.42 272.04 267 (2") 260.5 266.77 1870 9350 390 260.39 *544' 11 #3/ Kingsbury Dr.

LIFT STATION DESIGN FLOW CALCULATIONS & ELEVATIONS

*NOTE: FROM LIFT STATION TO MANHOLE WITH GRAVITY SEWER

LIFT STATION #2

ADJUSTABLE -

METAL STRAP

MANHOLE

SLEEVE (TYP.)

(TYP.)

MANHOLE -STEPS

> PITCH UP TO WALL. **PLAN** 12" OF 3/4" STONE 4 4 4 UNDISTURBED EARTH **SECTION**

STANDARD SEWER MANHOLE SECTION

-SHAPE SMOOTH ROUNDED INVERT FOR

SIDE ENTRANCE PIPES

NOT TO SCALE

(3" MINIMUM THICKNESS) IF ENCOUNTERED IF USED, SHALL REMAIN IN PLACE BELOW TOP OF PIPE ZONE TYPICAL SEWER TRENCH DETAIL NOT TO SCALE INCLUDES HOUSE SERVICES EXISTING PAVEMENT SHALL BE SAW CUT BEFORE TRENCH EXCAVATION

-RESURFACING

AS REQUIRED

SELECT

-COMPACTED

(EXCAVATED

MATERIAL OR

-THOROUGHLY COMPACT WITH

PNEUMATIC

BORROW AS

SPECIFIED)

BACKFILL

24" Ø C.I. MANHOLE FRAME AND COVER SET COVER SHALL HAVE THE AT FINISHED GRADE (LEBARON LT102 CA) WORD "SEWER" IN 3" LETTERS EMBOSSED ON FACE SET CASTING IN GROUT AND GROUT ALL AROUND - PLACE COUPLING OR ADJUST TO REQUIRED JOINT ON ALL PIPES WITHIN 3' OF WALL GRADE WITH A MINIMUM OF TWO COURSES OF BRICK MASONRY (10" MAX. HEIGHT) COPOLYMER POLYPROPYLENE PLASTIC PRECAST REINFORCED -BRICK SHELF OR ALUMINUM M.H. STEPS CONCRETE MANHOLE CONE AND INVERT SECTION 2', 3', OR 4' LENGTH BITUMINOUS DAMPROOFING TO BE APPLIED TO OUTER SURFACE (TYPICAL)

BUTYL RUBBER JOINT (TYP.) PRECAST REINFORCED CONCRETE MANHOLE BARREL (0.12 SQ. IN./L.F REINFORCEMENT PER ASTM & AASHTO SPECS) 3' MONOLITHIC

PRESSURE CAST BASE BRICK SHELF-INNER EDGE AT ELEVATION OF CROWN OF HIGHEST PIPE-1"

www.outback-ena.com